

# Assumptions Sea Level Rise and Datum

The Fort Point Channel Storm Surge Barrier concept was designed for consistency with Climate Ready Boston (CRB) proposed adaptations and analyzed based on sea level rise (SLR) projections in the Massachusetts Coastal Flood Risk Model (MC-FRM). The SLR values applied in MC-FRM are consistent with the standards for the State of Massachusetts developed by Coastal Zone Management. The MC-FRM utilizes a "High" SLR scenario. This scenario is based on the relative SLR projections under Relative Concentration Pathway (RCP) 8.5 (a "worst case scenario" of increasing atmospheric carbon concentrations) and represents elevations that have a 99.5% probability of not being exceeded within the respective timeframes. In 2030, that amounts to an increase of 1.3 feet in Boston from a baseline condition (2008 centered tidal epoch), and in 2070 that amounts to an increase of 4.3 feet.

The concept developed in this project was analyzed using coastal conditions that include 2070 projected SLR and storm surge resulting from a 100-year tropical storm. The peak water surface elevation (WSE) predicted by the MC-FRM during these conditions is approximately 13.8 feet NAVD88 (varies by location). In mid 2022, the Greater Boston Research Advisory Group (BRAG) issued an updated report with new SLR projections. The report acknowledges that long term SLR projections are associated with significant uncertainty, and that updated projections include less SLR by 2100 (compared to earlier projections in the 2015 BRAG Report). According to the report, the likely range of SLR by 2070 under an RCP 8.5 scenario is 1.4 - 2.8 feet. Based on this information, projections from the MC-FRM that were utilized in this project are conservative and appropriate for long term planning purposes.

Unless otherwise noted, all elevations are based on the NAVD88 vertical datum. Elevations given in NAVD88 can be converted to Boston City Base (BCB) elevation by adding 6.46 feet.



# *Climate Ready Boston and Shoreline Protection*

The Fort Point Channel Storm Surge Barrier concept was developed to maintain consistency with possible Climate Ready Boston (CRB) adaptations based on the latest available information at the time they were developed. As the CRB program continues to evolve, it is anticipated that proposed concepts will need to be adapted.

The concept was developed to be consistent with stated neighborhood design flood elevations. In the Fort Point Channel, where the stated design flood elevation is 15.5 feet, pumps and the storm surge barrier were designed to a minimum elevation of 15.5 feet.

At the time of this project, many CRB concepts were in early planning stages and not fully defined. In consideration of this, it was assumed the shoreline protection around the City of Boston is 100% effective for all modeling evaluations. This assumption eliminates overland coastal flooding from model predictions, allowing for isolation of flooding that results only from rainfall and stormwater that cannot be discharged due to high sea levels. It is important to recognize that additional flooding, beyond what is depicted herein, would be expected if 100% effective shoreline protection is not implemented.



# **Basis of Design**

### Storm Surge Barrier and Pump Station

The storage capacity of the FPC was estimated under present day and 2070 conditions based on 4.3 feet of sea level rise projected in the MC-FRM. The usable storage volume was calculated by developing a stage storage curve provided by BWSC and determining the volume between Mean Low Water (MLW) and the approximate upper rim elevation of the FPC (approximately 7.0 feet, NAVD88). For design and analysis purposes, a usable storage volume of 100 MG (2070 conditions) was assumed, based on previous bathymetric data provided by BWSC. Additional storage could be obtained by drawing down the water level (below the future low tide level) with the pumps. The present-day usable storage is approximately 160 MG. For the purpose of this analysis, it was assumed that the current-day mean low water is the lowest water surface elevation allowable within the channel; future geotechnical analysis may indicate that it is possible to pump to even lower water levels if needed.

The concept includes a storm surge barrier to isolate the Fort Point Channel with navigable gate structures. Two alternative navigable gate concepts were designed:

- Vertical lift gate alternative: Lower cost alternative, but higher visual impacts (see sheet 5). 1.
- Submerged axis flap gate alternative: Higher cost alternative with minimal visual impacts 2. (see sheet 6).

### **Rainfall and Coastal Conditions**

The Commission currently utilizes a 10-year, 24-hour design storm to establish its target level of service. For the purpose of sizing new piping and evaluating storage capacity, a projected 2070 10-year, 24-hour design storm was developed. For consistency with Climate Ready Boston, performance of the FPC SSC concept was also evaluated with projected rainfall from a 100-year tropical event (developed during the Commission's Inundation Model Project, as shown in Table 2. The FPC SSB was evaluated using a 100-year return period coastal boundary condition. Data for this condition were obtained from the MC-FRM. For the purpose of evaluating the effectiveness of the concept, it was further assumed that complete shoreline protection was implemented, preventing flow of water between the FPC tributary area and Boston Harbor. Table 3 contains a summary of the coastal conditions that were analyzed.

Scenario	Purpose	Rainfall Depth (in)	Peak Intensity (in/hr)
Present Day, 10- year, 24-hr design storm	Baseline Conditions	5.15	3.32
Projected 2070, 10- year, 24-hr design storm	Design Conditions	6.18	4.08
100-year Tropical Storm	Damage Analysis	9.58	0.84

### Table 3: Coastal Conditions

Scenario	Purpose	Peak Water Surface Elevation (ft, NAVD88)	Source
Present Day	Baseline Conditions	3.7	BWSC Existing Model (April 2016 Tide Cycle)
2070,100-year Tropical Storm	Damage Analysis	13.8	MC-FRM

### Table 2: Rainfall Conditions



# **Basis of Design**

### **Pump Station**

An analysis was conducted to determine required pumping capacity by simulating the 10-year, 24-hour design storm with 100-year tropical storm surge under 2070 conditions. The model was used to characterize the Hydraulic Grade Line (HGL) in tributary pipelines under existing conditions and evaluate different size pumps under the conditions described above. It was found that 500 CFS of pumping capacity is sufficient to maintain a WSE of 3.7 or less within the FPC during these conditions as shown in Figure 1.



Figure 1: Water Surface Elevation in FPC with 500 CFS Pumping Capacity

For the purpose of this evaluation, it was assumed that SSB gates were closed at low tide before the storm event and that the pumps were run to draw down the water level from the projected 2070 MLW elevation to the current MLW elevation to maximize storage. It was further assumed that gates remained closed throughout the storm event. During an actual storm event it may be possible to open the SSB gates when the exterior tide level drops below the water level in the basin.

Pumps were sized based on the SSB (and surrounding CRB flood barriers) having a crest elevation of 15.5 feet NAVD88 (for consistency with CRB), with a discharge centerline at 17.0 feet NAVD88. The pump station utilizes 3 duty and 2 standby pumps, each with a capacity of approximately 167 CFS and static head range of 21 feet (low water level) to 4 feet (high-high water level, basin overflowing).The area of the pump station is 5,700 ft<sup>2</sup>.

Several alternatives for the FPC pump station were analyzed including use of diesel engine driven pumps, electric submersible pumps, electric non-submersible pumps, and construction of an offsite station. The concept herein utilizes electric submersible pumps with a temporary offsite emergency power supply on the basis of minimized visual impacts and minimized permitting challenges.



# **Storm Surge Barrier and Navigation Gates**



### Vertical Lift Gate Alternative

The vertical lift gate alternative utilizes four 115' wide gate sections that are stowed in the air (above the storm surge barrier superstructure) when in the open position. A navigable passage is provided in addition to three auxiliary flow gates. When open, the storm surge barrier does not impede existing navigation. The gates can be lowered into the "closed" position ahead of an extreme storm event to isolate the FPC.

The vertical lift gate alternative is associated with high visual impacts and lower cost compared with the submerged axis flap gate alternative.



# **Storm Surge Barrier and Navigation Gates**



# Submerged Axis Flap Gate Alternative

The submerged axis flap gate alternative utilizes four 115' wide gate sections that are stored on the channel bottom recessed in a reinforced concrete pier foundation/sill. A navigable passage is provided in addition to three auxiliary flow gates. When open, the storm surge barrier does not impede existing navigation. The gates can be lifted into the "closed" position with telescoping hydraulic drive cylinders and trolley attached to the gate arm before an extreme storm event to isolate the FPC.

The submerged axis flap gate offers minimized viewshed impacts compared to the vertical lift gate alternative, but is associated with a greater O&M burden, additional complexity, and higher cost.



# **Flood Modeling and Damage Analysis**





Figure 3: Loss of GDP





The flood reduction benefits of the FPC SSB concept were evaluated using the Commission's 2D Inundation Model by simulating a 100-year tropical storm event with 2070 SLR and storm surge. The figures on the following page depict the peak flooding that was predicted in the drainage area tributary to the FPC with shoreline protection only and with the FPC SSB and tide gates on all vulnerable BWSC owned outfalls. An analysis of economic losses/physical impacts from flooding under both scenarios was performed by risQ Inc.

Model predictions indicate that the FPC SSB concept reduces physical damage by **\$3.1 billion**, avoids **\$15.9 billion** in rebuilding costs, and mitigates a GPD loss of **\$2.7** billion during a 100-year tropical storm event in 2070 compared to shoreline protection only.

Note: replacement values include the total value of impacted buildings in flooded areas (e.g., impacted buildings are fully replaced), whereas physical damage includes estimated costs to repair flood damage based on predicted flood depths and building characteristics. The values shown are the average of minimum and maximum calculated losses. Refer to the Project's Final Report for more information.

Fort Point Channel Concept





Flood Depth > 4 in and < 1 ft

Flood Depth > 1 ft and < 2 ft

Flood Depth

Drainage Area

### **Simulation Parameters**

Storm Type	100yr Tropical Storm
Rainfall Depth	9.6 inches
Peak WSE 2070 SLR + 100yr Surge	13.8 feet NAVD88

Coastal Stormwater Discharge Analysis Fort Point Channel

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November 2022

# **Cost Estimate and FEMA BRIC Considerations**

### **Capital Cost Estimate**

A construction cost estimate for the FPC SSB and pump station concept was developed for planning purposes. Assumptions made for the cost estimate include 15-year escalation to the mid-point of construction and the inclusion of a 50% design contingency. Installation and labor costs for the portable generator as well as hookup costs to existing electrical services were not included in this estimate.

### Table 4: Fort Point Channel Pump Station Cost Estimate Subtotals

Remaining Design Development & Construction Administration (assumed 20% of total less design contingency)	\$4,524,000
Direct Construction Costs	\$8,968,000
Indirect Construction Costs	\$1,794,000
Mark-Up (Including 50% design contingency)	\$22,760,000
Total	\$38,045,000

# Social Vulnerability and FEMA BRIC Funding

FEMA BRIC funding prioritizes disadvantaged communities. Table 6 contains a summary of several indicators for the Fort Point Channel tributary area that could be used help characterize the community for future FEMA funding applications and prioritization of projects that benefit disadvantaged communities.

### Table 6: Fort Point Channel Tributary Area Social Vulnerability Indicators

Low Income & Persistent Poverty		
Per Capita Income	\$58,326	
Below Poverty Line	23%	
High Housing Cost Burden		
Stressed Renters (>40% rent-to-income)	34%	
Households With Food Insecurity	14%	
Racial and Ethnic Segregation		
Asian Population	13%	
Black Population	30%	
Latino Population	20%	
White Population	41%	
Employment		
Adults Age 25+ Without High School (or equivalent) Degree	16%	
Unemployment Rate (Age 16+)		

Data provided by risQ inc. from the US census and American Community Survey

### Table 5: Fort Point Channel Marine Work Alternatives Cost Estimate Subtotals

	Submerged Axis Flap Gate	Submerged Axis Flap Gate – South Location	Vertical Lift Gate
Remaining Design Development & BWSC Construction Administration	\$60,553,000	\$49,851,000	\$36,350,000
Direct & Indirect Construction Costs Total (Marked-up)*	\$329,465,000	\$271,236,000	\$197,328,000
Escalation (15 Years)	\$240,119,000	\$197,682,000	\$143,867,000
Design Contingency (35%)	\$136,506,000	\$112,381,000	\$81,788,000
Total	\$766,643,000	\$631,150,000	\$459,333,000
* Marked-up cost includes contractor overhead, profit, fees, bond, insurance.			



# **Adaptability and Implementation**

# **Adaptability**

Figure 5 below depicts historical daily rainfall totals and tide levels. As shown in this figure, the conditions that were used to design and analyze the FPC concept are conservative and represent more extreme conditions than have occurred historically. It is important to note that the design storm generates higher intensity rainfall than is typically associated with tropical and nor'easter storm events. As such, it is likely that the FPC pump station is sufficiently large to discharge flows from most future rain events. Regardless, the following measures could be implemented to adapt the concept to more severe conditions (additional SLR, more intense rainfall, etc.) in the future:

- Convert a standby pump to a duty pump for additional capacity
- Increase the size of each FPC pump unit
- Redirect additional vulnerable outfalls to discharge into the FPC behind the • SSB



### Figure 5: Design and Analysis Conditions vs. Historical Tide and Rainfall

# Implementation Considerations

- Coordination with CRB (and other relevant stakeholders) to construct adequate shoreline • protection to prevent flanking of the SSB.
- Shoreline elevation projects along the interior of the FPC should still be evaluated since the FBC SSB is not designed to isolate the basin from "day-to-day" high tide levels.
- A thorough permitting evaluation of the FPC SSB should be conducted. Although the SSB concept was developed to maximize tidal exchange between the FPC and Boston Harbor, a of constructing a SSB.
- these structures. Construction of an "integrated" structure could reduce construction costs, minimize viewshed impacts, and provide additional public amenities.
- coordinate availability of backup power several days before an anticipated extreme storm event
- Deauthorization of the federally authorized navigation channel would be a significant undertaking. It is recommended that future development of this concept preserve the existing navigation function.
- the concept in the future
- 2D modeling was conducted to evaluate the incremental benefit of adding tide gates to all BWSC owned outfalls that discharge into the FPC in addition to shoreline elevation. It was found that additional tide gates have a substantial flood reduction benefit compared to shoreline elevation alone. Installation of tide gates on all BWSC owned outfalls could be considered as an interim measure before construction of the SSB and pump station.
- At a further stage of the design process, consideration should be given to mitigating the possible hazard caused by the high-flow discharges of the pump station to small boats, swimmers, and other harbor users.

thorough environmental impact assessment should be conducted to evaluate possible impacts

Coordination with the Northern Avenue Bridge replacement team could facilitate integration of

The FPC SSB pump station uses a temporary backup power supply; as such, it is important to

Additional outfalls could be redirected to discharge behind the FPC SSB and pump station to mitigate the effect of higher sea levels - this would increase the regional utility and benefits of

Coastal Stormwater Discharge Analysis Fort Point Channel **Boston Water and** Hazen wer Commission

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November 2022

# Storm Surge Barrier Northern Avenue Bridge Integrated Alternative



### **Conceptual Solution – Alternate Location**

The above graphic depicts an alternate concept for the FPC SSB that is integrated with the planned Northern Avenue Bridge Replacement. The mechanical systems are similar to the other concepts shown previously. This alternate location combines planned renovations to the Northern Avenue Pedestrian Bridge with the FPC SSB into one project. This alternative mitigates the viewshed impact of the SSB on the planned viewing platform of the bridge. Although this concept h not been advanced to the design stage at this time, and it is anticipated that both structures would need to be significantly redesigned, integration of these structures would reduce construction impacts from separate projects. In addition, an integrated project would offer an opportunity for Boston and the Commission to implement an iconic adaption project, with multiple community benefits, that could catalyze funding and coordination for further adaptation efforts.

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	Sheet 11 of 15	November 2022
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# **Replicability and Implementation Timeline**



## Summary of Similar Concepts

The map on this sheet depicts other vulnerable outfalls that could utilize elements of the FPC SSB solution

Several outfalls along the South Boston Waterfront could be consolidated with a new conduit and redirected to a new outfall behind the FPC SSB.

Outfalls near the Charles River Dam could be consolidated with new conduits and redirected to new outfalls behind the dam.

Outfalls at the "back" of the Reserved Channel (west of Summer Street) could be protected with a SSB.

Outfalls within the Little Mystic Channel could be protected with a SSB.

Additional information about these outfalls can be found in the Commission's Coastal Stormwater Discharge Analysis Implementation Timeline.



# ATTACHMENT A FORT POINT CHANNEL PUMP STATION CONCEPTUAL DESIGN DRAWINGS

- A-1: Storm Surge Barrier Overview Plan
- A-2: Pump Station Plan
- A-3: Pump Station Section View
- A-4: Electrical Single Line Diagram (Three Gate Alternative)
- A-5: Electrical Single Line Diagram (Four Gate Alternative)







# **Fort Point Channel Pump Station**



# NOTES

- 1. FOR WATER SURFACE ELEVATIONS REFER TO OPERATIONAL TABLE.
- 2. ALL ELEVATIONS USE THE NAVD88 VERTICAL DATUM UNLESS OTHERWISE STATED.
- 3. CONCEPTUAL DRAWING, NOT FOR CONSTRUCTION.

STORMWATER PUMP-1,2,3,4,5 OPERATIONAL PARAMETERS		
FLOW RATE, CFS	167	
STATIC HEAD RANGE, FT	3.5 - 23.0	
DESIGN FLOOD ELEVATION, FT 15.5		

### STORMWATER PUMP-1,2,3,4,5 OPERATIONAL WSE TABLE

NOTE	OPERATION	ELEVATION, FT
А	HIGH LEVEL ALARM	13
В	LAG PUMP ON	5.0
С	LEAD PUMP ON	-2.6
D	LEAD PUMP OFF	-3.6
E	LOW LOW ALARM	-4.6
G	MIN PUMP SUBMERGENCE	-5.6





# 

700A ) 250A ) 50A 480-120/208V 3PHASE UP-X BLE HVAC LIGHTING UNIT PANEL IP-X

> ELECTRICAL FLOOD WATER PUMP STATION PROPOSED SINGLE LINE DIAGRAM

### COASTAL STORMWATER DISCHARGE ANALYSIS





A-4

November 2022



A-5

November 2022

# ATTACHMENT B FORT POINT CHANNEL STORM SURGE BARRIER CONCEPTUAL DESIGN DRAWINGS VERTICAL LIFT GATE ALTERNATIVE

B-1: Vertical Lift Gate Plan

B-2: Vertical Lift Gate Phase 1 Plan

B-3: Vertical Lift Gate Phase 2 Plan

B-4: Vertical Lift Gate Phase 3 Plan

B-4: Vertical Lift Gate Phase 4 Plan

B-5: Vertical Lift Gate Phase 5 Plan

B-6: Vertical Lift Gate Top Surface and Foundation Plans

**B-7: Vertical Lift Gate Elevations** 

**B-8: Vertical Lift Gate Pier Section** 

**B-9: Vertical Lift Gate Sections and Details** 





### NOTES:

THE CONCEPT PLAN VIEWS SHOW PORTIONS OF STORM SURGE BARRIER STRUCTURE THAT PROJECT ABOVE DAILY HIGH WATER LINE. SEE S-400 SERIES SHEETS FOR MULTIVIEWS AND DETAILS ASSOCIATED WITH BELOW WATER AND CHANNEL BED FEATURES (FOUNDATION, SCOUR PROTECTION).

6

- 2. SHORELINE INTERFACES, TRANSITIONS, AND TIE-INS SHALL BE DETERMINED DURING DETAILED DESIGN.
- EXISTING SHORELINE STRUCTURES AND OBSTRUCTIONS TO BE REMOVED SHALL BE DETERMINED DURING DETAILED DESIGN.



6



### NOTES - PHASE 1:

THIS CONCEPT PHASING PLAN REPRESENTS ONE POSSIBLE CONFIGURATION THE CONTRACTOR COULD UTILIZE TO ESTABLISH IN-THE-DRY CONDITIONS TO CONSTRUCT THE SILL, PIERS, GATES, AND BALANCE OF SUPERSTRUCTURE. ACTUAL PHASING PROGRESSION, EXTENTS, AND INTERFACES SHALL BE DETERMINED BY THE FINAL DESIGN OF THE SURGE BARRIER AND CONTRACTOR'S COFFERDAM SYSTEM.

6

CONTRACTOR SHALL DREDGE TEMPORARY NAVIGATION CHANNEL AND PROVIDE TEMPORARY NAVIGATION AIDS PRIOR TO CONSTRUCTING PHASE 1 TEMPORARY COFFERDAM.



DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

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SCALE: 1"=80'

PRELIMINARY -

CONCEPT LEVEL



### NOTES - PHASE 2:

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DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

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DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

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### NOTE:

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DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

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![](_page_27_Figure_1.jpeg)

DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

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DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

# ATTACHMENT C FORT POINT CHANNEL STORM SURGE BARRIER CONCEPTUAL DESIGN DRAWINGS SUBMERGED AXIS FLAP GATE ALTERNATIVE

- C-1: Submerged Axis Flap Gate Plan
- C-2: Submerged Axis Flap Gate Phase 1 Plan
- C-3: Submerged Axis Flap Gate Phase 2 Plan
- C-4: Submerged Axis Flap Gate Top Surface and Foundation Plans
- C-5: Submerged Axis Flap Gate Elevations
- C-6: Submerged Axis Flap Gate (Stowed) Section
- C-7: Submerged Axis Flap Gate (Deployed) Section
- C-8: Submerged Axis Flap Gate Plan South Location Alternative
- C-9: Submerged Axis Flap Gate Phase 1 Plan South Location Alternative
- C-10: Submerged Axis Flap Gate Phase 2 Plan South Location Alternative
- C-11: Submerged Axis Flap Gate Top Surface and Foundation Plans South Location Alternative
- C-12: Submerged Axis Flap Gate Elevations South Location Alternative
- C-13: Submerged Axis Flap Gate (Stowed) Section South Location Alternative
- C-14: Submerged Axis Flap Gate (Deployed) Section South Location Alternative

![](_page_29_Picture_17.jpeg)

![](_page_30_Picture_0.jpeg)

### NOTES:

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![](_page_30_Figure_5.jpeg)

INDEX: 1 OF 7

6

SCALE: 1"=80'

PRELIMINARY -

CONCEPT LEVEL

DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

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![](_page_31_Figure_4.jpeg)

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![](_page_32_Picture_0.jpeg)

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SHORELINE INTERFACES, TRANSITIONS, AND TIE-INS SHALL BE DETERMINED DURING DETAILED DESIGN.

3. EXISTING SHORELINE STRUCTURES AND OBSTRUCTIONS TO BE REMOVED SHALL BE DETERMINED DURING DETAILED DESIGN .

![](_page_39_Figure_7.jpeg)

DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

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![](_page_40_Figure_0.jpeg)

DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

![](_page_41_Figure_0.jpeg)

DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

![](_page_42_Figure_0.jpeg)

DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING

![](_page_43_Figure_0.jpeg)

DRAWING SCALES SHOWN BASED ON 22"x34" DRAWING